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CIVL7008 Seismic Analysis for Building Structures

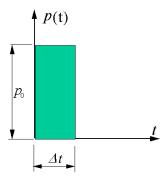
Lec-03 Forced Vibration of SDOF System





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Force Vibration: Rectangle Impulse Excitation



$$I = p_0 \Delta t$$

$$m\ddot{v} + c\dot{v} + kv = 0$$

$$v(t) = e^{-\xi\omega t} \left[\frac{\dot{v}_0 + v_0 \xi\omega}{\omega_d} \sin \omega_d t + v_0 \cos \omega_d t \right]$$

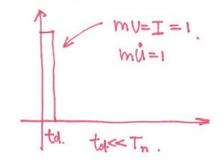
$$\dot{v}_0=0, \qquad \dot{v}_0=\frac{p_0}{m}\Delta t$$

$$v(t) = e^{-\xi \omega t} \frac{p_0 \Delta t}{m \omega_d} \sin \omega_d t$$



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Rectangular Impulse



$$\begin{cases} V(0) = \frac{1}{m} & \text{initial Condition} \\ U(0) = 0 \end{cases}$$

Mamped System.

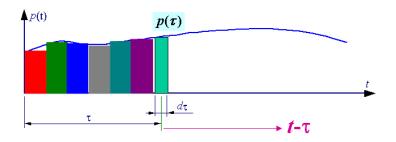
Patientar Solution

Up = St Sp(c) exp[-Sw(t-z)]. Sin and (t-z) dz

Duhamel Integration.



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From
$$v(t) = e^{-\xi \omega t} \frac{p_0 \Delta t}{m \omega_d} \sin \omega_d t$$

$$t \to t - \tau$$

$$p_0 \to p(\tau)$$

$$\Delta t \to d\tau$$

$$dv(t) = \frac{p(\tau) d\tau}{m \omega_d} e^{-\xi \omega(t - \tau)} \sin \omega_d (t - \tau)$$

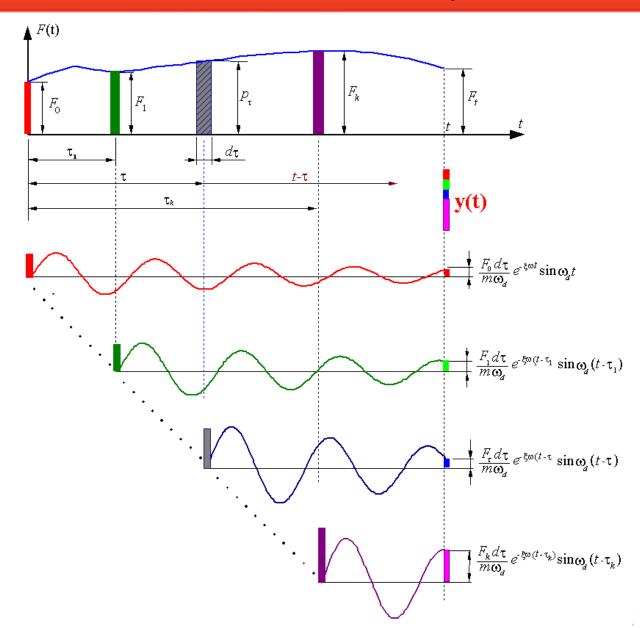
$$v(t) = \int_0^t \frac{p(\tau)}{m\omega_d} e^{-\xi\omega(t-\tau)} \sin \omega_d(t-\tau) d\tau$$



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Excel Table for Duhamel Integration

$$v(t) = A(t) \sin \omega_D t - B(t) \cos \omega_D t$$

$$A(t) \equiv \frac{1}{m \,\omega_{\scriptscriptstyle D}} \, \int_0^t \, p(au) \, \frac{\exp(\xi \omega au)}{\exp(\xi \omega t)} \, \cos \omega_{\scriptscriptstyle D} au \, d au$$

$$B(t) \equiv \frac{1}{m \,\omega_D} \, \int_0^t \, p(\tau) \, \frac{\exp(\xi \omega \tau)}{\exp(\xi \omega t)} \, \sin \omega_D \tau \, \, d\tau$$

Simple summation:

$$A_N \doteq A_{N-1} \exp(-\xi \omega \triangle \tau) + \frac{\triangle \tau}{m \omega_D} y_{N-1} \exp(-\xi \omega \triangle \tau)$$

$$N = 1, 2, 3, \cdots \quad (6-17a)$$

Trapezoidal rule:

$$A_N \doteq A_{N-1} \exp(-\xi \omega \triangle \tau) + \frac{\triangle \tau}{2 m \omega_D} \left[y_{N-1} \exp(-\xi \omega \triangle \tau) + y_N \right]$$

$$N = 1, 2, 3, \dots \quad (6-17b)$$

Simpson's rule:

$$A_{N} \doteq A_{N-2} \exp(-2 \xi \omega \triangle \tau)$$

$$+ \frac{\Delta \tau}{3 m \omega_{D}} \Big[y_{N-2} \exp(-2 \xi \omega \triangle \tau) + 4 y_{N-1} \exp(-\xi \omega \triangle \tau) + y_{N} \Big]$$

$$N = 2, 4, 5, \cdots \quad (6-17c)$$





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Excel Table for Duhamel Integration

TABLE E6-2 Numerical Duhamel integral analysis including damping

N	I _N	P _N kips (1)	sin30 <i>t</i> ₈	cos301 _A .	Y _K (1)×(3) kips (4)		y _{N-2}	M ₁ × (5)	M ₂ × [(6)+(9)] (8)	A _{N-2} F	\(\frac{A_N}{F}\) (4)+(7)+(8) (10)	1 1 1		У _{К-3}	(12)	M ₂ × [(13)+(16)] (15)	$\frac{B_{N-2}}{F}$ (16)	<u>B_N</u> F (11)+(14)+(15) (17)	(10)×(2) (18)	(17)×(3) (19)	(18)-(19) (20)	v _N F×(20) fi (21)	f _{S_N} k×(21) kips (22)
0	0.000	0	0	1.000	0	_	_	_			0	0	_	1	_	_	_	0	0	0	0	0	0
1	0.005	19.32	0.149	0.989	19.1	0	_		_	_	_	2.88	0	_	_	-		_		_	_	_	_
2	0.010	38.64	0.296	0.955	36.9	19.1	0	75.8	0	0	112.7	11.4	2.88	0	11.4	0	0	22.8	33.3	21.8	11.5	0.0002	0.58
3	0.015	57.96	0.435	0.900	52.2	36.9	19.1	_	_	_	_	25.2	11.4	2.88		-	_	-	_	_	_	_	
4	0.020	77.28	0.565	0.825	63.8	52.2	36.9	207.2	147.4	112.7	418.4	43.7	25.2	11.4	100.0	33.7	22.8	177.4	236	146	90	0.0017	4.50
5	0.025	96.60	0.682	0.732	70.7	63.8	52.2	_	-		_	65.9	43.7	25.2	-	_	_		_	_	-	_	***
6	0.030	77.28	0.783	0.622	48.1	70.7	63.8	280.7	475.0	418.4	803.8	60.5	65.9	43.7	261,6	217.8	177.4	539.9	629	336	293	0.0054	14.65
7	0.035	57.96	0.867	0.498	28.9	48.1	70.7	_	_	-	-	50.3	60.5	65.9	-	_	_	_	-	-	-	_	
8	0.040	38,64	0,932	0.362	14.0	28.9	48.1	114.7	839.1	803.8	967.8	36.0	50.3	60.5	199.7	591.4	539.9	827.1	902	299	603	0.0112	30.2
9	0.045	19.32	0.976	0.219	4.23	14.0	28.9	-	_	_	_	18.9	36.0	50.3	-	-	-	-	-	-	_	_	
10	0.050	0	0.997	0.0707	0	4.23	14.0	16.8	967.1	967.8	983.9	0	18.9	36.0	75.0	850.1	827.1	925.1	981	65.4	915	0.0169	45.8
11	0.055	0	0.997	-0.0791	0	0	4.23	-	-	_	_	0	0	18.9	-	_		-	-	-	_	-	
12	0.060	0	0.974	-0.227	0	0	0	0	969.1	983.9	969.1	0	0	0	0	911.2	925.1	911.2	900	-206	1106	0.0205	55.4
13	0.065	0	0.929	-0.370	0	0	0	-	-	-	-	0	0	0		_	_	-	-	-	-	_	-
14	0.070	0	0.863	-0.505	0	0	0	0	954.6	969.1	0	0	0	0	0	897.5	911.2	897.5	824	-453	1277	0.0236	63.9
15	0.075	0	0.778	-0.628	0	0	0	-	-	-	_	0	0	0	_	_		_	-	-	_	-	-
16	0.080	0	0.675	-0.737	0	0	0	0	940.3	954.6	940.3	0	0	0	0	884.0	897.5	884.0	635	-651.5	1286	0.0238	64.3
17	0.085	0	0.558	-0.830	0	0	0	_	_	-		0	0	0	_	_		-	-	-	_	-	
18	0.090	0	0.427	-0.904	0	0	0	0	926.2	940.3	926.2	0	0	0	0	870.7	884.0	870.7	395	-787	1182	0.0219	59.1

$$\omega = \sqrt{\frac{kg}{W}} = 30 \, rad \, / \sec \quad \Delta \tau = 0.005 \sec \quad M_1 = 4 \exp \left(-\xi \omega \Delta \tau \right) = 3.97 \quad M_2 = \exp \left(-2 \, \xi \omega \Delta \tau \right) = 0.985 \quad F = \frac{\Delta \tau}{3 m \omega} = 1852 \times 10^{-5} \, ft \, / \, kip \quad k = 2700 \, kips \, / \, ft$$

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Excel Table for Duhamel Integration

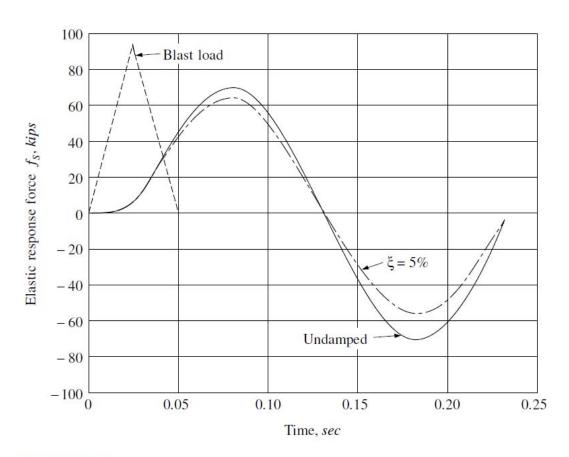


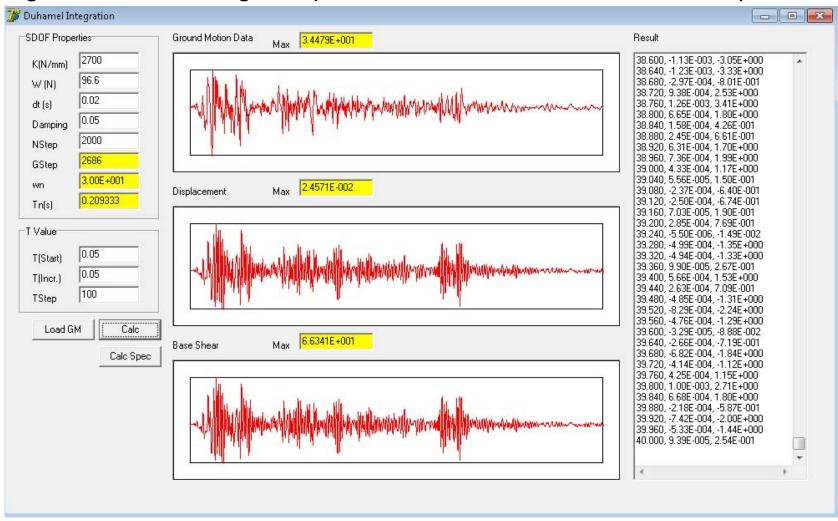
FIGURE E6-2 Response of water tower to blast load.





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Program for Duhamel Integration (Solver For Force Vibration Problem of SDOF)

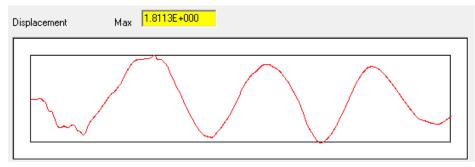




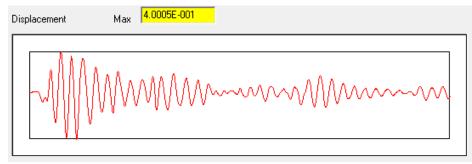


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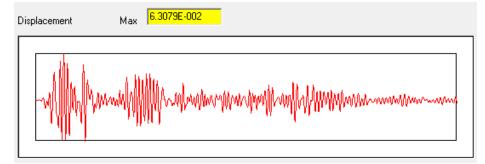
K = 1, T1 = 10.87 s

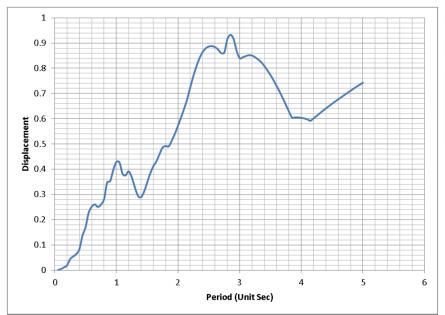


K = 100, T1 = 1.087 s



K = 1000, T1 = 0.344 s





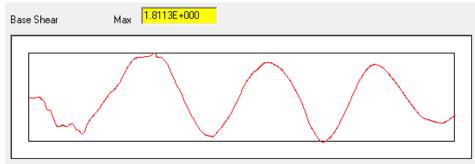




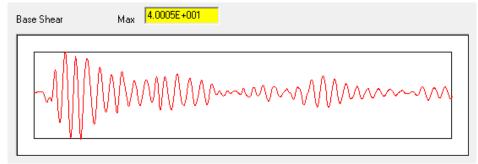
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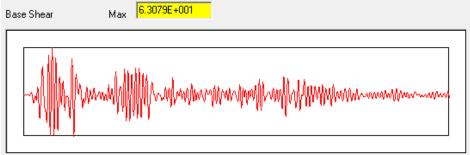
K = 1, T1 = 10.87 s

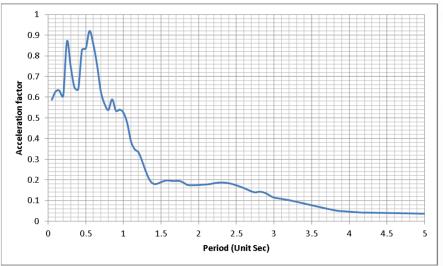


K = 100, T1 = 1.087 s



K = 1000, T1 = 0.344 s





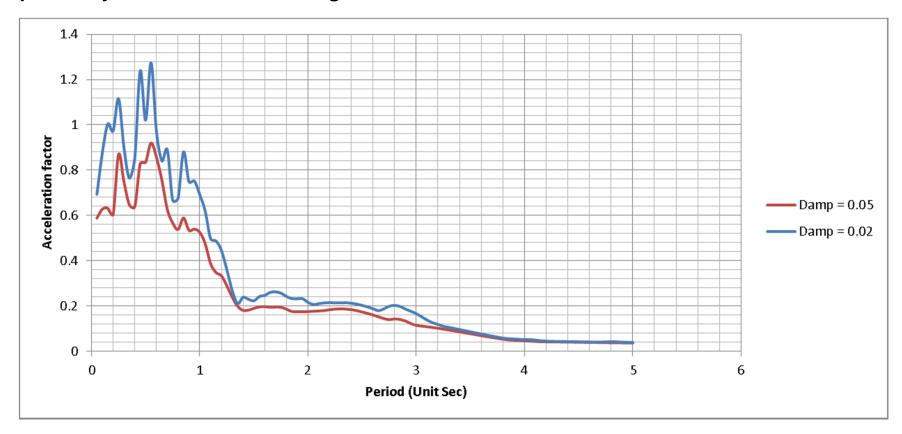
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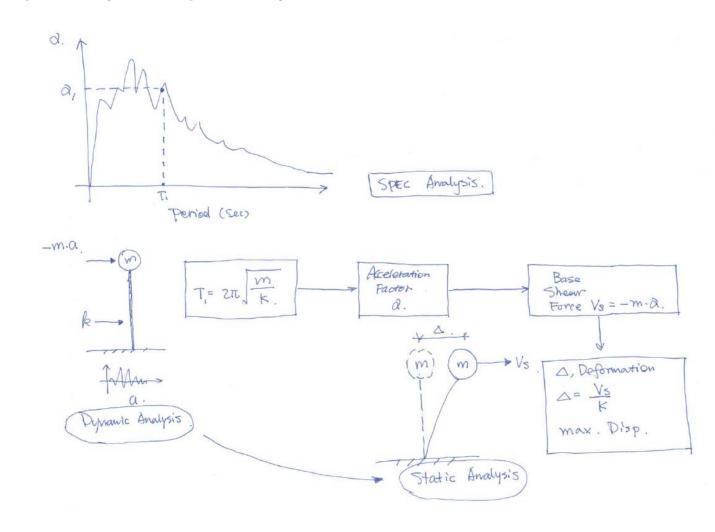
Spec Analysis From Duhamel Integration





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Spec Analysis Simple Theory

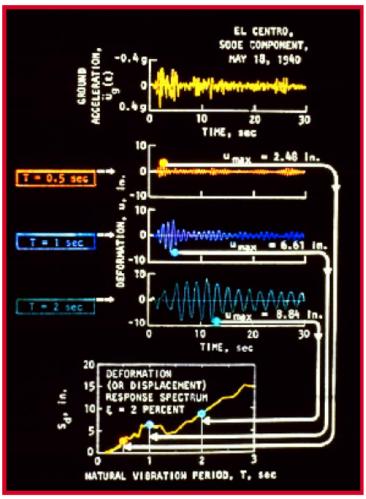


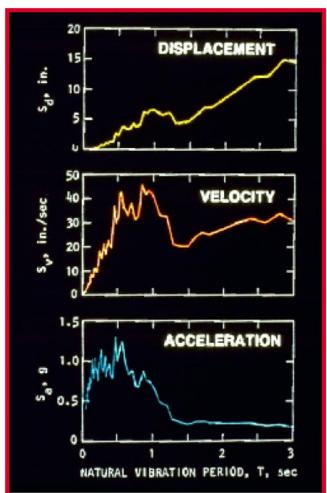




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Spec Analysis Simple Theory



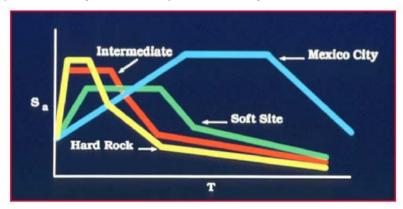


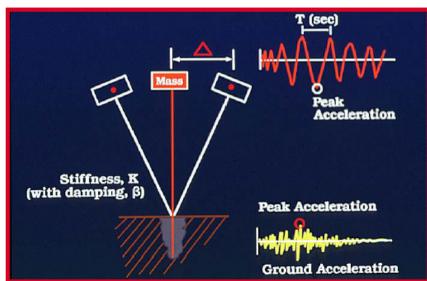


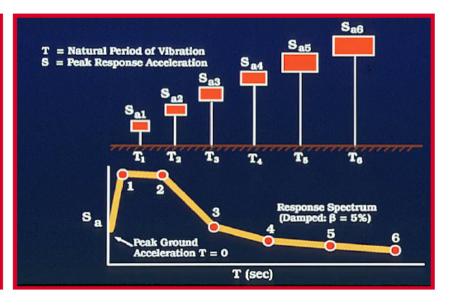


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Spec Analysis Simple Theory





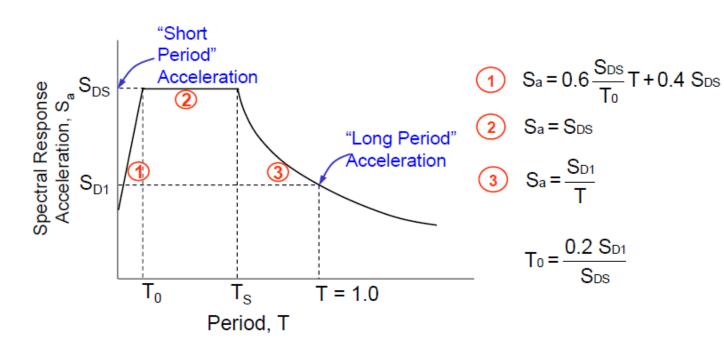






Spec Analysis Simple Theory

ASCE 7-02 Uses a Smoothed Design Acceleration Spectrum





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SDOF Simulation Program

Lec-03 Forced Vibration of SDOF System



